



SYSTEMS ENGINEERING & RESEARCH ENVIRONMENTAL DESIGN AND SYSTEMS ANALYSIS . INDUSTRIAL WASTE MANAGEMENT

Wastewater Collection System Study

For

Proposed **Destin Landing** 760 Acre Development Located in the

SW ¼ Sec. 10, T8N, R2W, IM E ½ Sec. 15, T8N, R2W, IM S. ½ Sec. 14, T8N, R2W, IM Norman, OK

Prepared By:

Search, Inc. 6730 E. Cedar Lane Norman, OK 73026

January 21, 2015

Revised March 23, 2015



6730 E. Cedar Lane • Norman, Oklahoma 73026-5516 Phone (405) 364-0900 streebin@seachinc.us

Table of Contents

Title Page	1
Table of Contents	2
Introduction	3
Volume and Strength of Sewage	3
Existing System	5
Project Description	5
Project Cost Estimates	Pending
Conclusions and Recommendations	7

Attachment #1 – Calculations of Average Daily Flows and Peak Hourly Flows Attachment #1-1 Service Area Sec. 14 Attachment #1-2 Service Area NE Sec. 15 Attachment #1-3 Service Area Sec. 10 Attachment #1-4 Service Area Sec. S. 15 Attachment #1-5 Service Area Existing Lift Station Post Oak Road

Attachment #2 – Pressure Loss and System Curve Calculations Attachment #2-1 8" PVC Force Main LS Sec. 14 to LS NE Sec. 15 Attachment #2-3 8" PVC Force Main LS NE Sec. 15 to LS NE Sec. 10 Attachment #2-3 12" PVC Force Main LS Sec. 10 to 24" Gravity Sewer

Exhibit #1 – Developer Supplier Master Plan

Exhibit #2 – Drainage Area and Land Usage Map

Exhibit #3 – Overall Layout

Exhibit #4 – Preliminary Force Main and Hydraulic Profile

Exhibit #5 – Example of Typical Lift Station

Wastewater Collection Study Proposed Destin Landing Development in SE Norman, OK

Introduction

The purpose of this study is to determine the feasibility of providing a wastewater collection system to serve Destin Landing, a proposed 760 acre Residential/Commercial Development located in South East Norman, Oklahoma. Specifically the development is located in the SW ¼ Sec. 10, T8N, R2W; E ½ Sec. 15, T8N, R2W; and S. ½ Sec. 14, T8N, R2W, IM, Cleveland County. For exact location see Exhibit #1 – Developer Supplied Master Plan.

The development will contain single family homes, multi-family homes, service areas, parks, and commercial areas supporting the community. This development is divided into five natural drainage basins. Each of these five basins will need a method of transporting wastewater generated into the City of Norman's wastewater collection system. This study will develop a plan for transporting this waste water to the City of Norman's wastewater collection system.

Volume and Strength of Sewage

The first step will be to determine the volume and strength of the wastewater in each drainage basin. Since the proposed development is residential and commercial there will be no industrial or other high strength wastes. Constituent concentrations in the wastewater generated will be consistent with typical municipal sanitary wastewater. The City of Norman's Wastewater Treatment system will not be impacted by the concentration, pH or temperature of the wastewater generated in this development.

The drainage basins or service areas were defined by placing the Master Plan Land Uses on top of a 7.5 Min. USGS Topo map. The drainage basins were outlined in black and the land use in each tabulated. The resulting map is shown in Exhibit #2 – Drainage Area and Land Use Map. The map shows each drainage basin and the total area of each proposed land use in the basins.

The average daily wastewater flow for each drainage basin is based on the Land Uses provided by the Developer in the Master Plan (Exhibit #1). The average daily wastewater flow was calculated using the area of each land use in the basin, density of

housing units and the average number of residents in each unit. The areas of each land uses is estimated on the Master Plan. The density of the housing units and the average number of residents in each unit were based on reference data and experience.

As an example the average wastewater flow for area #1 for Service Area Sec. 10 would be calculated as follows. Area #1 is 36.5 acres of Single Family – High Density homes. Reference data and experience tell us that the Unit Density is 3.5 units/ac and that an average of 3.0 people live in each home. The Oklahoma Department of Environmental Qualities' regulations dictate that all wastewater collection and treatment system will be designed based on a wastewater flow of 100 gallons/capita/day (gpcpd). The average daily wastewater flow for this one area would be 36.5 ac x 5 units/ac x 3.0 people per home x 100 gpcpd = 54,750 gpd. In Attachment #1 – Calculations of Average Daily Flows and Peak Hourly Flows, and the total average daily flow for each drainage basin/service area have been calculated.

The peak wastewater flow for each basin has been calculated using a Peaking Factor of 4.0. This factor was provided by the Norman Utilities Authority and is used throughout the City. This Peaking Factor establishes a ratio of Peak Hourly Flow to the Design Average Flow. At the bottom of each table in Attachment #1 the ratio has been determined for each service area and the peak hourly flow determined. Below is a summary table for the Average Daily Flow and Peak Hourly Flow for each service area.

Service Area	Approx. Total	Average Daily	Peaking	Peak Hourly
	Area (ac)	Flow (gpd)	Factor	Flow (gpm)
Sec. 14	280	164,630	4.0	460
Sec. NE 15	100	114,085	4.0	320
Sec. 10	240	346,140	4.0	970
Subtotal	620	654,855		1,750
Sec S 15	90	83,690	4.0	240
Existing Post	50	52,800	4.0	150
Oak LS				
Total	760	761,345		2,140
Development				

Table #1 – Service Area Summary Table

After the development is fully constructed the average daily flow entering the City of Norman's wastewater collection system will be 761,345 gpd. The peak hourly flow will be 2,140 gpm.

gravity sewer. (Exhibit #3) The other two lift stations would be connected into this force main along this route. The total length of this force main is approximately 16,400 feet.

The profile for the proposed route is shown in Exhibit #4. Head loss and friction calculations are given in Appendix #2. The size of the force main was selected based on two criteria, a minimum flushing velocity of 2 ft/sec. and minimizing friction and head loss. An 8" force main was selected to run from LS Sec 15 NE to LS Sec 10 and the size was increased to a 12" from LS Sec 10 to the 24" gravity sewer.

The headloss for the force main is summarized in the table below.

Flow (gpm)	320	460	780	970	1290	1430	1750
LS Sec 14 to LS	NA	45.9	NA	NA	NA	NA	NA
Sec 15 NE							
LS Sec 15 NE to	0.0	0.0	10.5	NA	NA	NA	NA
LS Sec 10							
LS Sec 10 to 24"	18.3	21.5	32.2	40.8	58.8	68.1	92.2
Gravity							
Suction Lift	15.0	15.0	15.0	15.0	15.0	15.0	15.0
Total Head (ft)							
Sec. 14	NA	82.4	NA	NA	NA	NA	NA
Sec 14, and Sec	NA	NA	103.6	NA	NA	NA	NA
15NE							
Sec 14, Sec 15	NA	NA	NA	NA	NA	NA	163.6
NE, and Sec 10							
Sec 14, and Sec	NA	NA	NA	NA	NA	129.0	NA
10							
Sec 10	NA	NA	NA	55.8	NA	NA	NA
Sec 10, Sec 15	NA	NA	NA	NA	84.3	NA	NA
NE							
Sec 15 NE	33.3	NA	NA	NA	NA	NA	NA

Table 2 – Headloss Summary (ft)

The use of a common force main will require slightly larger pumps to insure the peak flows can be maintained. The use of variable frequency drives will allow the pumps to operate during non-peaks without overloading.

In order to maintain minimum flushing velocities in the common force main the lift stations must be constructed in the following order Lift Station Sec 10, Lift Station 15 NE

and finally, Lift Station Sec 14. Lift Stations Sec 15NE and Sec14 will not have the flow to maintain the minimum flushing velocities in the 12 inch forcemain from Lift Station Sec 10 to the 24 inch gravity sewer.

All four lift stations will be of similar design. The stations will be standard duplex or triplex lift stations with variable frequency drives, submersible pumps, and emergency generators. A drawing of a typical triplex lift station is included in Exhibit #5. This Exhibit is for reference and only intended to show the type of lift station proposed for this development.

Project Cost Estimate

Pending

Conclusions and Recommendations

This development is a long term project that will be completed over several years. The wastewater collection system must be flexible and constructed on a as need basis. The proposed solution of four lift stations and a common force main will allow the system to be constructed as needed.

The recommended wastewater collection system is based on available information. Final designs will be based on updated and actual data.

Attachment #1

Calculations of Average Daily Flows and Peak Hourly Flows

Attachment #1-1 Service Area Sec. 14 Attachment #1-2 Service Area NE Sec. 15 Attachment #1-3 Service Area Sec. 10 Attachment #1-4 Service Area Sec. S. 15 Attachment #1-5 Service Area Existing Lift Station Post Oak Road

Basin	Area	and	Usages	(ac)	
Baom	/	aa	obugoo	(00)	

Drainage Basin #	Equestrian Facility	Service	Commerical / Mixed Use	Open Space	Multi- Family	Single Family Estate	Single Family Low	Single Family Med	Single Family High	Patio Homes	Total
Dwelling Density (units/acre)		13			15	1	3	4	5	6	
Dwelling Population (people/dwelling)		1.5			2.5	3	3	3	3	2.5	
PE/ac	5	19.5	10	0.5	37.5	3	9	12	15	15	
Flow per Person (gpdpc)		100			100	100	100	100	100	100	
Flow per Unit (gpd)	500	150	1000	50	250	300	300	300	300	250	
Flow per Acre (gpapd)	500	1950	1000	50	3750	300	900	1200	1500	1500	
Sec. 14 - 1	0.0	0.0	0.0	0.0	0.0	10.3	0.0	0.0	0.0	0.0	10.3
Sec. 14 - 2	0.0	0.0	0.0	0.0	0.0	7.1	0.0	0.0	0.0	0.0	7.1
Sec. 14 - 3	0.0	0.0	0.0	0.0	0.0	21.0	0.0	0.0	0.0	0.0	21.0
Sec. 14 - 4	0.0	0.0	0.0	0.0	0.0	20.4	0.0	0.0	0.0	0.0	20.4
Sec. 14 - 5	0.0	0.0	0.0	0.0	0.0	22.6	0.0	0.0	0.0	0.0	22.6
Sec. 14 - 6	3.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	3.8
Sec. 14 - 7	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	20.5	0.0	20.5
Sec. 14 - 8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	21.3	0.0	0.0	21.3
Sec. 14 - 9	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	24.0	0.0	24.0
Sec. 14 - 10	0.0	0.0	0.0	0.0	5.8	0.0	0.0	0.0	0.0	0.0	5.8
Sec. 14 - 11	0.0	0.0	10.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	10.0
Sec. 14 - 12	0.0	0.0	0.0	0.0	28.7	0.0	0.0	0.0	0.0	0.0	28.7
T	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
i otal	3.8	0.0	10.0	0.0	34.5	81.4	0.0	21.3	44.5	0.0	195.5
PE	19	0	100	0	1,294	244	0	256	668	0	2,580

Drainage Basin	Equestrian		Commerical /		Multi-	Single	Single	Single	Single	Patio	Total Flow
	Equestinan	Service	Mixed Llee	Open Space	Fomily	Family	Family	Family	Family	Homes	per Basin
#	Facility		wixed Use		ганиу	Estate	Low	Med	High	High	(gpd)
Sec. 14 - 1	0	0	0	0	0	3,090	0	0	0	0	3,090
Sec. 14 - 2	0	0	0	0	0	2,130	0	0	0	0	2,130
Sec. 14 - 3	0	0	0	0	0	6,300	0	0	0	0	6,300
Sec. 14 - 4	0	0	0	0	0	6,120	0	0	0	0	6,120
Sec. 14 - 5	0	0	0	0	0	6,780	0	0	0	0	6,780
Sec. 14 - 6	1,900	0	0	0	0	0	0	0	0	0	1,900
Sec. 14 - 7	0	0	0	0	0	0	0	0	30,750	0	30,750
Sec. 14 - 8	0	0	0	0	0	0	0	25,560	0	0	25,560
Sec. 14 - 9	0	0	0	0	0	0	0	0	36,000	0	36,000
Sec. 14 - 10	0	0	0	0	0	0	0	0	36,000	0	36,000
Sec. 14 - 11	0	0	10,000	0	0	0	0	0	0	0	10,000
Sec. 14 - 12	0	0	0	0	107,625	0	0	0	0	0	107,625
0	0	0	0	0	0	0	0	0	0	0	0
Total	1,900	0	10,000	0	0	24,420	0	25,560	102,750	0	164,630

Flow Peaking Factor, includes I&I

cludes I&I 4.0

Peak Flow (gpm) 457

Design Flow 460

Basin	Area	and	Usages	(ac)	
Baom	/	aa	obugoo	(00)	

Drainage Basin #	Equestrian Facility	Service	Commerical / Mixed Use	Open Space	Multi- Family	Single Family Estate	Single Family Low	Single Family Med	Single Family High	Patio Homes	Total
Dwelling Density (units/acre)		13			15	1	3	4	5	6	
Dwelling Population (people/dwelling)		1.5			2.5	3	3	3	3	2.5	
PE/ac	5	19.5	10	0.5	37.5	3	9	12	15	15	
Flow per Person (gpdpc)		100			100	100	100	100	100	100	
Flow per Unit (gpd)	500	150	1000	50	250	300	300	300	300	250	
Flow per Acre (gpapd)	500	1950	1000	50	3750	300	900	1200	1500	1500	
Sec. 15 NE - 1	0.0	0.0	10.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	10.0
Sec. 15 NE - 2	0.0	0.0	4.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	4.1
Sec. 15 NE - 3	0.0	0.0	0.0	0.0	9.2	0.0	0.0	0.0	0.0	0.0	9.2
Sec. 15 NE - 4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	2.2	2.2
Sec. 15 NE - 5	0.0	8.7	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	8.7
Sec. 15 NE - 6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	11.2	11.2
Sec. 15 NE - 7	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	10.5	0.0	10.5
Sec. 15 NE - 8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	8.3	0.0	8.3
Sec. 15 NE - 9	0.0	0.0	0.0	4.4	0.0	0.0	0.0	0.0	0.0	0.0	4.4
Total	0.0	8.7	14.1	4.4	9.2	0.0	0.0	0.0	18.8	13.4	68.6
PE	0	170	141	2	345	0	0	0	282	201	1,141

Drainage Basin	Equestrian		Commerical /		Multi-	Single	Single	Single	Single	Patio	Total Flow
#	Facility	Service	Mixed Llee	Open Space	Family	Family	Family	Family	Family	Homes	per Basin
#	raciiity		WINEU USE		1 anniy	Estate	Low	Med	High	High	(gpd)
Sec. 15 NE - 1	0	0	10,000	0	0	0	0	0	0	0	10,000
Sec. 15 NE - 2	0	0	4,100	0	0	0	0	0	0	0	4,100
Sec. 15 NE - 3	0	0	0	0	34,500	0	0	0	0	0	34,500
Sec. 15 NE - 4	0	0	0	0	0	0	0	0	0	3,300	3,300
Sec. 15 NE - 5	0	16,965	0	0	0	0	0	0	0	0	16,965
Sec. 15 NE - 6	0	0	0	0	0	0	0	0	0	16,800	16,800
Sec. 15 NE - 7	0	0	0	0	0	0	0	0	15,750	0	15,750
Sec. 15 NE - 8	0	0	0	0	0	0	0	0	12,450	0	12,450
Sec. 15 NE - 9	0	0	0	220	0	0	0	0	0	0	220
0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0
Total	0	16,965	14,100	220	34,500	0	0	0	28,200	20,100	114,085

Flow Peaking Factor,

includes I&I 4.0

Peak Flow (gpm) 317

Design Flow 320

Basin Area and	Usages	(ac)
----------------	--------	------

Drainage Basin #	Equestrian Facility	Service	Commerical / Mixed Use	Open Space	Multi- Family	Single Family Estate	Single Family	Single Family Med	Single Family High	Patio Homes	Total
						Lotate	LOW	Mica	riigii		
Dwelling Density (units/acre)		13			15	1	3	4	5	6	
Dwelling Population (people/dwelling)		1.5			2.5	3	3	3	3	2.5	
PE/ac	5	19.5	10	0.5	37.5	3	9	12	15	15	
Flow per Person (gpdpc)		100			100	100	100	100	100	100	
Flow per Unit (gpd)	500	150	1000	50	250	300	300	300	300	250	
Flow per Acre (gpapd)	500	1950	1000	50	3750	300	900	1200	1500	1500	
Sec. 10 - 1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	36.5	0.0	36.5
Sec. 10 - 2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	11.0	0.0	11.0
Sec. 10 - 3	0.0	0.0	3.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	3.2
Sec. 10 - 4	0.0	0.0	0.0	5.3	0.0	0.0	0.0	0.0	0.0	0.0	5.3
Sec. 10 - 5	0.0	0.0	0.0	0.0	17.8	0.0	0.0	0.0	0.0	0.0	17.8
Sec. 10 - 6	0.0	0.0	0.0	0.0	12.2	0.0	0.0	0.0	0.0	0.0	12.2
Sec. 10 - 7	0.0	0.0	0.0	0.0	8.5	0.0	0.0	0.0	0.0	0.0	8.5
Sec. 10 - 8	0.0	0.0	3.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	3.8
Sec. 10 - 9	0.0	0.0	0.0	0.0	16.1	0.0	0.0	0.0	0.0	0.0	16.1
Sec. 10 - 10	0.0	0.0	0.0	0.0	11.4	0.0	0.0	0.0	0.0	0.0	11.4
Sec. 10 - 11	0.0	0.0	2.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0	2.5
Sec. 10 - 12	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	13.9	13.9
Sec. 10 - 13	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	25.5	0.0	25.5
Total	0.0	0.0	9.5	5.3	66.0	0.0	0.0	0.0	73.0	13.9	167.7
PE	0	0	95	3	2,475	0	0	0	1,095	209	3,876

Drainage Basin	Equestrian		Commerical /		Multi-	Single	Single	Single	Single	Patio	Total Flow
	Equility	Service	Mixed Llee	Open Space	Fomily	Family	Family	Family	Family	Homes	per Basin
#	Facility		wixed Use		ганиу	Estate	Low	Med	High	High	(gpd)
Sec. 10 - 1	0	0	0	0	0	0	0	0	54,750	0	54,750
Sec. 10 - 2	0	0	0	0	0	0	0	0	16,500	0	16,500
Sec. 10 - 3	0	0	3,200	0	0	0	0	0	0	0	3,200
Sec. 10 - 4	0	0	0	265	0	0	0	0	0	0	265
Sec. 10 - 5	0	0	0	0	66,750	0	0	0	0	0	66,750
Sec. 10 - 6	0	0	0	0	45,750	0	0	0	0	0	45,750
Sec. 10 - 7	0	0	0	0	31,875	0	0	0	0	0	31,875
Sec. 10 - 8	0	0	3,800	0	0	0	0	0	0	0	3,800
Sec. 10 - 9	0	0	0	0	60,375	0	0	0	0	0	60,375
Sec. 10 - 10	0	0	0	0	60,375	0	0	0	0	0	60,375
Sec. 10 - 11	0	0	2,500	0	0	0	0	0	0	0	2,500
Sec. 10 - 12	0	0	0	0	0	0	0	0	0	20,850	20,850
Sec. 10 - 13	0	0	0	0	0	0	0	0	38,220	0	38,220
Total	0	0	9,500	265	265,125	0	0	0	71,250	0	346,140

Flow Peaking Factor, includes I&I

Peak Flow (gpm) 962

Design Flow 970

Drainage Basin #	Equestrian Facility	Service	Commerical / Mixed Use	Open Space	Multi- Family	Single Family Estate	Single Family Low	Single Family Med	Single Family High	Patio Homes	Total
Dwelling Density (units/acre)		13			15	1	3	4	5	6	
Dwelling Population (people/dwelling)		1.5			2.5	3	3	3	3	2.5	
PE/ac	5	19.5	10	0.5	37.5	3	9	12	15	15	
Flow per Person (gpdpc)		100			100	100	100	100	100	100	
Flow per Unit (gpd)	500	150	1000	50	250	300	300	300	300	250	
Flow per Acre (gpapd)	500	1950	1000	50	3750	300	900	1200	1500	1500	
Sec. 15 S - 1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	18.7	0.0	0.0	18.7
Sec. 15 S - 2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	14.9	0.0	14.9
Sec. 15 S - 3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	22.8	22.8
Sec. 15 S - 4	0.0	0.0	4.7	0.0	0.0	0.0	0.0	0.0	0.0	0.0	4.7
Total	0.0	0.0	4.7	0.0	0.0	0.0	0.0	18.7	14.9	22.8	61.1
PE	0	0.0	47	0.0	0	0	0	224	224	342	837

Drainage Basin	Equestrian	Service	Commerical /	Open Space	Multi-	Single Family	Single Family	Single Family	Single Family	Patio Homes	Total Flow per Basin
#	Facility		Mixea Use		Family	Estate	Low	Med	High	High	(gpd)
Sec. 15 S - 1	0	0	0	0	0	0	0	22,440	0	0	22,440
Sec. 15 S - 2	0	0	0	0	0	0	0	0	22,350	0	22,350
Sec. 15 S - 3	0	0	0	0	0	0	0	0	0	34,200	34,200
Sec. 15 S - 4	0	0	4,700	0	0	0	0	0	0	0	4,700
0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	4,700	0	0	0	0	22,440	22,350	34,200	83,690

Flow Peaking Factor,

includes I&I 4.0

Peak Flow (gpm) 232

Design Flow 240

Attachment #1-5

Basin Area and Usages (ac)

					•	· /					
Drainage Basin #	Equestrian Facility	Service	Commerical / Mixed Use	Open Space	Multi- Family	Single Family Estate	Single Family Low	Single Family Med	Single Family High	Patio Homes	Total
Dwelling Density (units/acre)		13			15	1	3	4	5	6	
Dwelling Population (people/dwelling)		1.5			2.5	3	3	3	3	2.5	
PE/ac	5	19.5	10	0.5	37.5	3	9	12	15	15	
Flow per Person (gpdpc)		100			100	100	100	100	100	100	
Flow per Unit (gpd)	500	150	1000	50	250	300	300	300	300	250	
Flow per Acre (gpapd)	500	1950	1000	50	3750	300	900	1200	1500	1500	
Ex. Post Oak - 1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	9.9	0.0	9.9
Ex. Post Oak - 2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	25.3	0.0	25.3
Total	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	35.2	0.0	35.2
PE	0	0	0	0	0	0	0	0	528	0	528

Basin Ultimate Flows (gpd)

Drainage Basin #	Equestrian Facility	Service	Commerical / Mixed Use	Open Space	Multi- Family	Single Family Estate	Single Family Low	Single Family Med	Single Family High	Patio Homes High	Total Flow per Basin (gpd)
Ex. Post Oak - 1	0	0	0	0	0	0	0	0	14,850	0	14,850
Ex. Post Oak - 2	0	0	0	0	0	0	0	0	37,950	0	37,950
0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	52,800	0	52,800

Flow Peaking Factor,

includes I&I 4.0

Peak Flow (gpm) 147

Design Flow

150

Attachment #2

Pressure Loss and System Curve Calculations

Attachment #2-1 8" PVC Force Main LS Sec. 14 to LS NE Sec. 15 - NA Attachment #2-2 8" PVC Force Main LS NE Sec. 15 to LS NE Sec. 10 Attachment #2-3 12" PVC Force Main LS Sec. 10 to 24" Gravity Sewer- NA

System Curve LS Sec. 14 - LS Sec. 15 NE Norman, Oklahoma

AWWA D900 - DR 14 - Class 200 PVC

Nominal Pip	be Size	8							
ID (in.)		7.758		Suction Head	1105				
OD (in.)		9.05		Discharge Head	1125				
SDR		12.01		Other	0				
Area (ft^2)		0.33		Static Head	20				
Chw		140.00							
# of Heads		0						Total	System
Increment		20.00				Total	Minor	Dynamic	Curve
Flow		Flow	Velocity	Velocity Head	Friction Loss	Friction Loss	Losses	Head	
(gpm)		(ft^3/s)	(ft/s)	(ft)	(ft/100ft.)	(ft/100ft.)	(ft)	(ft)	(ft)
	0								20
	200.00	0.4456	1.358	0.029	0.094	5.35	0.17	5.5	25.5
	220.00	0.4902	1,493	0.035	0.112	6.38	0.21	6.6	26.6
	240.00	0.5348	1.629	0.041	0.132	7.49	0.25	7.7	27.7
	260.00	0.5793	1.765	0.048	0.153	8.69	0.29	9.0	29.0
	280.00	0.6239	1.901	0.056	0.175	9.97	0.34	10.3	30.3
	300.00	0.6684	2.036	0.064	0.199	11.33	0.39	11.7	31.7
	320.00	0.7130	2.172	0.073	0.224	12.76	0.44	13.2	33.2
	340.00	0.7576	2.308	0.083	0.251	14.28	0.50	14.8	34.8
	360.00	0.8021	2.444	0.093	0.279	15.87	0.56	16.4	36.4
	380.00	0.8467	2.579	0.103	0.308	17.55	0.62	18.2	38.2
	400.00	0.8913	2.715	0.114	0.339	19.29	0.69	20.0	40.0
	420.00	0.9358	2.851	0.126	0.371	21.12	0.76	21.9	41.9
	440.00	0.9804	2.987	0.139	0.405	23.02	0.83	23.9	43.9
	460.00	1.0250	3.122	0.151	0.439	24.99	0.91	25.9	45.9
	480.00	1.0695	3.258	0.165	0.475	27.04	0.99	28.0	48.0
	500.00	1.1141	3.394	0.179	0.513	29.17	1.07	30.2	50.2
	520.00	1.1586	3.530	0.193	0.551	31.37	1.16	32.5	52.5
	540.00	1.2032	3.665	0.209	0.591	33.64	1.25	34.9	54.9
	560.00	1.2478	3.801	0.224	0.632	35.98	1.35	37.3	57.3
	580.00	1.2923	3.937	0.241	0.675	38.39	1.44	39.8	59.8

Length of Line				
Minor Losses	к	Ν	lumber	
90 deg. bend		0.3	4	1.20
45 deg. Bend		0.17	0	0.00
22.5 deg. Bend		0.1	0	0.00
Gate Valve		0.1	2	0.20
Check Valve		2	1	2.00
Tee side		0.6	1	0.60
Tee Through		0.15	0	0.00
Entrance		1	1	1.00
Exit		1	1	1.00
Foot Valve		0.8	0	0.00
Basket Strainer		1	0	0.00



System Curve LS Sec. 15 NE - LS Sec. 10 Norman, Oklahoma

AWWA D900 - DR 14 - Class 200 PVC

Nominal Pi	oe Size	8							
ID (in.)		7.758		Suction Head	1138				
OD (in.)		9.05		Discharge Head	1120				
SDR		12.01		Other	0				
Area (ft^2)		0.33		Static Head	-18				
Chw		140.00							
# of Heads		0						Total	System
Increment		40.00				Total	Minor	Dynamic	Curve
Flow		Flow	Velocity	Velocity Head	Friction Loss	Friction Loss	Losses	Head	
(gpm)		(ft^3/s)	(ft/s)	(ft)	(ft/100ft.)	(ft/100ft.)	(ft)	(ft)	(ft)
	0								-18
	200.00	0.4456	1.358	0.029	0.094	2.09	0.17	2.3	-15.7
	240.00	0.5348	1.629	0.041	0.132	2.92	0.25	3.2	-14.8
	280.00	0.6239	1.901	0.056	0.175	3.89	0.34	4.2	-13.8
	320.00	0.7130	2.172	0.073	0.224	4.98	0.44	5.4	-12.6
	360.00	0.8021	2.444	0.093	0.279	6.19	0.56	6.7	-11.3
	400.00	0.8913	2.715	0.114	0.339	7.53	0.69	8.2	-9.8
	440.00	0.9804	2.987	0.139	0.405	8.98	0.83	9.8	-8.2
	460.00	1.0250	3.122	0.151	0.439	9.75	0.91	10.7	-7.3
	500.00	1.1141	3.394	0.179	0.513	11.38	1.07	12.5	-5.5
	540.00	1.2032	3.665	0.209	0.591	13.12	1.25	14.4	-3.6
	580.00	1.2923	3.937	0.241	0.675	14.98	1.44	16.4	-1.6
	620.00	1.3815	4.208	0.275	0.763	16.95	1.65	18.6	0.6
	660.00	1.4706	4.480	0.312	0.857	19.03	1.87	20.9	2.9
	700.00	1.5597	4.751	0.351	0.956	21.22	2.10	23.3	5.3
	740.00	1.6488	5.023	0.392	1.059	23.52	2.35	25.9	7.9
	780.00	1.7380	5.294	0.435	1.168	25.93	2.61	28.5	10.5
	820.00	1.8271	5.566	0.481	1.281	28.44	2.89	31.3	13.3
	860.00	1.9162	5.837	0.529	1.399	31.07	3.17	34.2	16.2
	900.00	2.0053	6.109	0.579	1.522	33.80	3.48	37.3	19.3
	940.00	2.0945	6.380	0.632	1.650	36.63	3.79	40.4	22.4

К	Ν	lumber		
	0.3	4		1.20
	0.17	0	(0.00
	0.1	0	(0.00
	0.1	2	().20
	2	1		2.00
	0.6	1	(0.60
	0.15	0	(0.00
	1	1		1.00
	1	1		1.00
	0.8	0	(0.00
	1	0	(0.00
	к	K N 0.3 0.17 0.1 0.1 2 0.6 0.15 1 1 0.8 1	2220 K Number 0.3 4 0.17 0 0.1 0 0.1 2 2 1 0.6 1 0.15 0 1 1 1 1 0.8 0 1 0	2220 K Number 0.3 4 0.17 0 0.17 0 0.1 0 0.1 0 0.1 0 0.1 0 0.1 0 0.1 0 0.1 0 0.1 0 0.1 0 0.6 1 0.6 1 0.15 0 0.8 0 1 0



System Curve LS Sec. 10 - MH Norman, Oklahoma

AWWA D900 - DR 14 - Class 200 PVC

Nominal Pi	pe Size	12							
ID (in.)		11.314		Suction Head	1110				
OD (in.)		13.2		Discharge Head	1125				
SDR		12.00		Other	0				
Area (ft^2)		0.70		Static Head	15				
Chw		140.00							
# of Heads		0						Total	System
Increment		100.00				Total	Minor	Dynamic	Curve
Flow		Flow	Velocity	Velocity Head	Friction Loss	Friction Loss	Losses	Head	
(gpm)		(ft^3/s)	(ft/s)	(ft)	(ft/100ft.)	(ft/100ft.)	(ft)	(ft)	(ft)
	0								15
	100.00	0.2228	0.319	0.002	0.004	0.37	0.01	0.4	15.4
	200.00	0.4456	0.838	0.006	0.015	1.34	0.04	1.4	16.4
	300.00	0.6684	0.957	0.014	0.032	2.84	0.09	2.9	17.9
	320.00	0.7130	1.021	0.016	0.036	3.20	0.10	3.3	18.3
	420.00	0.9358	1.340	0.028	0.059	5.29	0.17	5.5	20.5
	460.00	1.0250	1.468	0.033	0.070	6.26	0.20	6.5	21.5
	560.00	1.2478	1.787	0.050	0.101	9.01	0.30	9.3	24.3
	660.00	1.4706	2.106	0.069	0.136	12.21	0.41	12.6	27.6
	700.00	1.5597	2.234	0.077	0.152	13.62	0.46	14.1	29.1
	780.00	1.7380	2.489	0.096	0.186	16.64	0.58	17.2	32.2
	800.00	1.7825	2.553	0.101	0.195	17.44	0.61	18.0	33.0
	900.00	2.0053	2.872	0.128	0.242	21.69	0.77	22.5	37.5
	970.00	2.1613	3.096	0.149	0.278	24.92	0.89	25.8	40.8
	1100.00	2.4510	3.511	0.191	0.351	31.45	1.15	32.6	47.6
	1200.00	2.6738	3.830	0.228	0.413	36.95	1.37	38.3	53.3
	1290.00	2.8743	4.117	0.263	0.472	42.25	1.58	43.8	58.8
	1430.00	3.1863	4.564	0.323	0.571	51.13	1.94	53.1	68.1
	1500.00	3.3422	4.787	0.356	0.624	55.86	2.14	58.0	73.0
	1600.00	3.5651	5.106	0.405	0.703	62.95	2.43	65.4	80.4
	1750.00	3.8993	5.585	0.484	0.830	74.31	2.91	77.2	92.2

Length of Line				
Minor Losses	К	1	Number	
90 deg. bend		0.3	4	1.20
45 deg. Bend		0.17	0	0.00
22.5 deg. Bend		0.1	0	0.00
Gate Valve		0.1	2	0.20
Check Valve		2	1	2.00
Tee side		0.6	1	0.60
Tee Through		0.15	0	0.00
Entrance		1	1	1.00
Exit		1	1	1.00
Foot Valve		0.8	0	0.00
Basket Strainer		1	0	0.00











WEST SIDE OF 36TH AVE.



SOUTH SIDE OF HWY 9

PARALLEL TO EAST SIDE OF 24TH AVE.



Drawing for Review only unless Signature and Date are Originals









- 1. LIFT STATION 2. LIFT STATION ACCESS HATCH
- W/ ORANGE SAFETY GRATE
- 3. VALVE VAULT 4. VALVE VAULT ACCESS HATCH
- W/ ORANGE SAFETY GRATE
- 5. SECURITY FENCE
- 6. STANDBY GENERATOR
- 7. ELECTRICAL SERVICE MASTER METER
- 8. BREAKER BOX
- 9. AUTO TRANSFER SWITCH
- 10. CONTROL PANEL
- 11. 240 BREAKER PANEL
- 12. 480/240 TRANSFORMER
- 13. TRANSFORMER DISCONNECT 14. MASTER DISCONNECT



Drawing for Review only unless Signature and Date are Originals